

FINAL

DESIGN DRAWINGS

STANDARD

*** SAFETY FIRST *** 1.0 DESIGN CRITERIA: 3.0 FOUNDATIONS 5.4 BOLTED CONNECTIONS SHALL CONFORM TO RCSC'S "SPECIFICATION FOR STRUCTURAL JOINTS USING ASTM A325 OR A490 BOLTS". ALL BOLTS SHALL BE 3/4" DIAMETER UNLESS OTHERWISE NOTED. US Army Corps A. BUILDING CODES AND SPECIFICATIONS: 3.1 SEE CIVIL DRAWINGS AND SPECIFICATIONS (PART OF SITE ADAPTION) FOR EARTHWORK of Engineers PREPARATION OF FOUNDATIONS INCLUDING THE REMOVAL OF ORGANIC MATERIALS, COMPACTING 5.5 WELDED CONNECTIONS SHALL CONFORM TO AWS D1.1 "STRUCTURAL WELDING CODE-STEEL". MINIMUM Huntsville Center 1. INTERNATIONAL BUILDING CODE 2009 (IBC) AS MODIFIED BY UFC 1-200-01 SOILS BENEATH STRUCTURES, BACK FILL REQUIREMENTS FOR OVER EXCAVATION AND REMOVAL OF SIZE FILLET WELDS SHALL BE 3/16" UNLESS OTHERWISE NOTED AND ELECTRODES SHALL BE E70xx. WELDERS SHALL BE QUALIFIED IN ACCORDANCE WITH AWS. 2. AMERICAN CONCRETE INSTITUTE (ACI 318) UNSUITABLE MATERIALS. 3. AMERICAN INSTITUTE OF STEEL CONSTRUCTION 4. AMERICAN WELDING SOCIETY, A.W.S. 3.2 MAXIMUM ASSUMED NET SOIL BEARING PRESSURE USED FOR DESIGN: 3000 PSF 5.6 UNLESS SPECIFICALLY DETAILED ON THE CONTRACT DRAWINGS. ALL FRAMED BEAM CONNECTIONS SHALL BE DESIGNED BY A QUALIFIED PROFESSIONAL ENGINEER EMPLOYED BY THE FABRICATOR. 3.3 ASSUMED UNIT WEIGHT OF SOIL USED FOR DESIGN: 120 PCF STANDARD BEAM CONNECTIONS (NON-COMPOSITE) SHALL BE DESIGNED BASED ON A REACTION B. LIVE LOADS EQUAL TO ONE-HALF THE MAXIMUM TOTAL UNIFORM LOAD CAPACITY FROM AISC'S "MAXIMUM TOTAL 3.4 ALL FOUNDATION BEARING SURFACES SHALL BE REVIEWED BY THE GEOTECHNICAL UNIFORM LOAD" TABLE MULTIPLIED BY A FACTOR OF 1.2, UNLESS REACTIONS ARE SHOWN ON ROOF---ENGINEER PRIOR TO PLACING CONCRETE TO ENSURE THEIR COMPLIANCE WITH THE STRUCTURAL DRAWINGS. MINIMUM REACTION TO DESIGN FOR SHALL BE (12.0 KIPS). FLOOR--PRESSURES NOTE ABOVE. 5.7 ALL EXTERIOR STEEL EXPOSED TO THE WEATHER SHALL BE HOT DIPPED GALVANIZED, UON. SNOW LOAD: 3.5 ALL FOOTINGS SHALL PROJECT AT LEAST 1'-6" INTO UNDISTURBED NATURAL SOIL OR MEMBERS NOT REQUIRED FOR CORROSION PROTECTION SHALL RECEIVE ONE COAT OF STANDARD GROUND SNOW LOAD (Pg) = 60 PSF COMPACTED ENGINEERED FILL HAVING A SOIL BEARING PRESSURE THAT MEETS OR PRIMER PAINT. DO NOT PRIME OR PAINT SURFACES WHICH ARE TO RECEIVE FIELD WELDED HEADED IMPORTANCE FACTOR (I) = 1.1 EXCEEDS THAT SPECIFIED ABOVE. SHEAR STUDS. PROVIDE 3" MINIMUM CONCRETE COVER FOR ALL STEEL BELOW GRADE AND PAINT EXPOSURE CATEGORY (Ce) = 1.0 WITH 2 COATS OF COAL TAR EPOXY. EPOXY SHALL MEET THE REQUIREMENTS OF PAINT SPECIFICATION SSPC-PAINT 16. 3.6 ALL DISTURBED EARTH UNDER FOOTINGS SHALL BE REPLACED WITH LEAN CONCRETE. THERMAL CATEGORY (Ct) = 1.0 3.7 CONCRETE SHALL NOT BE PLACED OVER FROZEN SOIL OR FOOTING EXCAVATIONS 5.8 ALL STIFFENERS AND GUSSETS PLATES SHALL BE MINIMUM 3/8" THICK, UNLESS OTHERWISE NOTED. C. WIND LOAD: SUBJECTED TO WATER. BASIC WIND SPEED: 130 MPH IMPORTANCE FACTOR (I): 1.15 6.0 LIGHTNING PROTECTION SYSTEM (LPS) EXPOSURE CATEGORY: C 4.0 CONCRETE **ENCLOSURE CLASSIFICATION: ENCLOSED** 6.1 ALL METAL PARTS, TO INCLUDE REINFORCEMENT IN FLOOR, WALLS/ARCH, LOUVERS, VENTILATOR, 4.1 ALL CONCRETE WORK INCLUDING DETAILING, FABRICATION, PLACEMENT OF REINFORCING, MIXING, HANDLING, DOOR AND DOOR FRAME, ETC., SHALL BE MADE ELECTRICALLY CONTINUOUS BY BONDING PLACING, FINISHING, AND CURING SHALL CONFORM TO THE FOLLOWING DOCUMENTS: D. EARTHQUAKE: PER NFPA 780 APPROVED METHOD(S) AT 5 LINEAR FEET INTERVALS. ELECTRICAL CONTINUITY SHALL BE PROVIDED ACROSS FLOOR EXPANSION AND ISOLATION JOINTS TO ARCH/WALL REINFORCEMENT ACI 301----"STANDARD SPECIFICATIONS FOR STRUCTURAL CONCRETE" OCCUPANCY CATEGORY=III AND SHALL BE PROVIDED DURING CONSTRUCTION. SEE ELECTRICAL DRAWINGS FOR ADDITIONAL ACI 315-----"MANUAL OF STANDARD PRACTICE FOR DETAILING REINFORCED CONCRETE STRUCTURES" INFORMATION REGARDING LPS. ACI 318-----"BUILDING CODE REQUIREMENTS FOR STRUCTURAL CONCRETE" Ss= 1.0 Sds = 0.67S1 = 0.44.2 ALL CONCRETE SHALL HAVE A MINIMUM COMPRESSIVE STRENGTH OF 4000 PSI AT 28 DAYS, U.O.N. Sd1 = 0.37ALL CONCRETE SHALL CONFORM TO ASTM C94. SITE CLASS: C BASIC SEISMIC-FORCE RESISTING SYSTEM: 4.3 REINFORCING BARS SHALL BE DEFORMED TYPE CONFORMING TO ASTM A615 GRADE 60 U.O.N. SPECIAL REINFORCED CONCRETE SHEARWALLS 4.4 WELDED WIRE REINFORCEMENT SHALL CONFORM TO ASTM A185. MINIMUM LAP AND EMBEDMENT TO BE THE SEISMIC DESIGN CATEGORY= D ANALYSIS PROCEDURE = EQUIVALENT LATERAL FORCE PROCEDURE GREATER OF ONE CROSS WIRE SPACING PLUS 2" OR 6", WHICHEVER IS GREATER. E. SOILS 4.5 FABRICATE AND PROVIDE BAR SUPPORTING ACCESSORIES IN ACCORDANCE WITH ACI MANUAL OF STANDARD PRACTICE AND C.R.S.I. SPECIFICATIONS. REINFORCING SHALL NOT BE WELDED IN ANY MANNER U.O.N. IN CONSTRUCTION DOCUMENTS. SOIL DENSITY (y): 120 PCF ANGLE OF INTERNAL FRICTION OF THE SOIL (ϕ): 30 DEGREES 4.6 REINFORCING SPLICES SHALL BE CONFORM WITH ACI 318 CLASS "B" TENSION LAP SPLICES, U.O.N. NO SPLICES AT-REST PRESSURE (EFP): 60 PSF PER FOOT OF DEPTH ACTIVE PRESSURE (EFP): 40 PSF PER FOOT OF DEPTH ARE PERMITTED IN THE PILASTERS AND DOOR HEADER. SPLICES IN THE 12" HEADWALL, IF NECESSARY, SHALL BE STAGGERED ON OPPOSITE FACES AND SPLICES OF ADJACENT PARALLEL BARS WITHIN THE SAME LAYER SHALL BE STAGGERED. STAGGER 12" HEADWALL SPLICES BY THE SPLICE LENGTH INDICATED IN THE DRAWINGS. 2.0 GENERAL 4.7 CONCRETE COVERAGE OF REINFORCEMENT FOR CAST-IN-PLACE CONSTRUCTION U.O.N.: 2.1 CONTRACTOR SHALL VERIFY ALL DIMENSIONS AND SITE CONDITIONS PRIOR TO CONCRETE CAST AGAINST EARTH:. CONSTRUCTION/FABRICATION. CONTRACTOR SHALL NOTIFY CONTRACTING OFFICER OF ANY DISCREPANCIES PRIOR TO CONSTRUCTION. FORMED CONCRETE EXPOSED TO EARTH OR WEATHER: NO. 6 BAR AND LARGER. DESIGNER NOTES: TO BE REMOVED WHEN PREPARING CONSTRUCTION DRAWINGS 2.2 THE STRUCTURE (MEMBERS AND CONNECTIONS) HAS BEEN DESIGNED TO SUPPORT NO. 5 BAR AND SMALLER. .1 1/2 INCHES FOR SITE ADAPTION OF THIS DESIGN. IN-PLACE DESIGN LOADS ONLY. THE CONTRACTOR SHALL BE RESPONSIBLE FOR LIMITING CONCRETE NOT EXPOSED TO WEATHER: CONSTRUCTION LOADS SUCH THAT THESE LOADS DO NOT EXCEED THE DESIGN LOADS SLABS, WALLS, JOISTS.,1 INCHES . THE MAGAZINE HAS BEEN ANALYZED FOR THE LOADS LISTED ON THIS SHEET AND DETERMINED NOTED ABOVE. BEAMS AND COLUMNS.. ..1 1/2 INCHES TO BE ADEQUATE UNDER THESE LOADINGS. HOWEVER, THE DESIGNER SHOULD VERIFY THE SLAB ON GRADE. .MID-DEPTH OF SLAB STRUCTURE FOR THE SITE-SPECIFIC LOADING CRITERIA. IF SITE-SPECIFIC LOADS EXCEED 2.3 IT IS THE CONTRACTOR'S RESPONSIBILITY TO DETERMINE CONSTRUCTION METHODS. THESE LISTED ON THIS SHEET, THE DESIGNER SHOULD ADDRESS ALL DEFICIENCIES THAT PROCEDURES, AND SEQUENCES TO ENSURE STABILITY AND SAFETY DURING CONSTRUCTION. 4.8 PROVIDE REINFORCING BARS IN CONCRETE FOOTINGS TO MATCH THE SIZE AND SPACING OF THE HORIZONTAL ARMY CORPS OF I ENGINEERING / SUPPORT CENT DO NOT MEET CURRENT BUILDING CODES. THE CONTRACTOR SHALL TAKE NECESSARY PRECAUTIONS TO PROTECT AND MAINTAIN THE REINFORCING AT ALL CORNERS AND INTERSECTIONS OF STRIP FOOTINGS. PROVIDE LEG LENGTH EQUIVALENT STRUCTURAL INTEGRITY OF ALL NEW AND EXISTING CONSTRUCTION AT ALL STAGES. TO CLASS "A" TENSION LAP SPLICE U.O.N. . FOUNDATIONS SHALL BE REVISED TO REFLECT SPECIFIC SITE SOIL CONDITIONS INCLUDING LOCAL SITING, TOPOGRAPHIC CONDITIONS, AND FROST PENETRATION DEPTHS. FINAL DESIGN 2.4 SECTIONS AND DETAILS SHOWN ON THE STRUCTURAL DRAWINGS SHALL BE CONSIDERED 4.9 PROVIDE DOWEL TO FOUNDATION WITH 90 DEGREE HOOK TO MATCH SIZE AND SPACING OF VERTICAL OF DRAINAGE SYSTEM AND WATERPROOFING AROUND MAGAZINE IS THE RESPONSIBILITY OF TYPICAL FOR SIMILAR CONDITIONS THAT DO NOT HAVE A SPECIFIC SECTION INDICATED. REINFORCING AT ALL PEDESTALS, WALLS, AND COLUMNS, THE SITE -ADAPT DESIGNER OF RECORD. 2.5 THE CONTRACTOR SHALL COORDINATE STANDARD DRAWINGS WITH THE VENDOR/MANF. 4.10 FOOTINGS AND SLABS SHALL HAVE NO HORIZONTAL JOINTS (POURED TO THEIR FULL DEPTHS IN STRUCTURAL COMPONENTS, WITH THE EXCEPTION OF THE FOUNDATION (FOOTINGS), ONE OPERATION). ANY STOP IN CONCRETE WORK SHALL BE BULKHEAD AND KEYED. U.O.N. SHOP DRAWINGS TO VERIFY SIZES AND LOCATIONS OF OPENINGS, SLEEVES, INSERTS, SLAB-ON-GRADE, AND WING WALLS SHALL NOT BE MODIFIED WITHOUT THE APPROVAL OF DEPRESSIONS, FINISHES, SLOPES, ETC. ANY DISCREPANCY SHALL BE BROUGHT TO THE CONTRACTING OFFICER, WHO SHOULD CONSULT WITH THE U.S. ARMY ENGINEERING THE ATTENTION OF THE CONTRACTING OFFICER. 4.11 REINFORCEMENT SHALL NOT BE BENT OR STRAIGHTENED IN A MANNER THAT WILL DAMAGE THE AND SUPPORT CENTER, HUNTSVILLE (STRUCTURAL BRANCH). STRUCTURE HAS BEEN MATERIAL. BARS WITH WITH KINKS OR IMPROPER BENDS SHALL NOT BE USED. DETERMINED TO BE ADEQUATE FOR THE DESIGN CRITERIA LISTED ON THIS SHEET. 2.6 SEE CIVIL SITE LAYOUT DRAWINGS (PART OF SITE ADAPTION) FOR ACTUAL FINISHED FLOOR ELEVATIONS (F.F.E.) FOR ALL BUILDINGS. ELEVATIONS SHOWN IN STRUCTURAL DOCUMENTS 4.12 REINFORCEMENT SHALL BE CONTINUOUS THROUGH ALL CONSTRUCTION JOINTS, BUT DISCONTINUOUS 1. SHEETS S701 - S705 (HIGH SECURITY HASP) AND S701(A) - S705(A) (ILD) IDENTIFY TWO WILL BE BASED ON REFERENCED F.F.E. EQUAL TO 100'-0", U.O.N. THROUGH ALL CONTROL JOINTS, U.O.N.. DIFFERENT LOCKING SYSTEMS. THE DESIGNER SHALL VERIFY WITH THE CONTRACTING OFFICER THE CORRECT LOCKING SYSTEM REQUIRED AND REMOVE THE REDUNDANT 2.7 ANY DISCREPANCIES BETWEEN DRAWINGS, SPECIFICATIONS, REFERENCE STANDARDS, OR 4.13 REFER TO GEOTECHNICAL REPORT FOR RECOMMENDATIONS RELATIVE TO SUBGRADE PREPARATION SHEETS FROM THE CONSTRUCTION CONTRACT DOCUMENTS FOR THE SYSTEM NOT USED. GOVERNING CODE, THE MORE STRINGENT REQUIREMENTS SHALL GOVERN. CONTRACTOR FOR SLAB ON GRADE WORK. SHALL NOTIFY THE CONTRACTING OFFICER OF DISCREPANCIES AND OBTAIN DIRECTION PRIOR TO PROCEEDING. 5.0 STRUCTURAL STEEL STRUCTURAL DESIGNATION (7-BAR) NOTES: 2.8 CONTRACTOR SHALL PROVIDE TEMPORARY SHORING AND BRACING OF ALL STRUCTURAL WORK, AND SOIL EXCAVATION AS REQUIRED. SHORING AND BRACING SHALL NOT BE 5.1 STRUCTURAL STEEL FABRICATION, ERECTION, AND CONNECTION DESIGN SHALL CONFORM TO 1. ANY DEVIATION FROM THE STANDARD APPROVED DESIGN DRAWINGS FOR THE CONCRETE REMOVED UNTIL ALL FINAL CONNECTIONS HAVE BEEN COMPLETED IN ACCORDANCE WITH A.I.S.C.'S "SPECIFICATION FOR STRUCTURAL STEEL BUILDINGS." HEADWALL, STEEL DOOR, CONCRETE ROOF OR THEIR SUPPORTS WITHOUT WRITTEN THE DRAWINGS, AND MATERIALS HAVE ACHIEVED DESIGN STRENGTH. APPROVAL FROM THE DEPARTMENT OF DEFENSE EXPLOSIVE SAFETY BOARD (DDESB) 5.2 STRUCTURAL STEEL SHALL CONFORM TO THE FOLLOWING ASTM DESIGNATIONS: MAY REQUIRE THE MAGAZINE TO BE CONSIDERED AN UNDEFINED MAGAZINE AND MAY 2.9 HEAVY COMPACTION AND OPERATION EQUIPMENT SHALL BE KEPT A MINIMUM DISTANCE SEVERELY RESTRICT THE ALLOWABLE STORAGE CAPACITY. EQUAL TO THE HEIGHT OF THE BACKFILL ABOVE THE FOUNDATION FROM THE BACK FACE W AND S SHAPES. .ASTM A36 OF THE REINFORCED CONCRETE WINGWALLS. THE CONTRACTOR SHOULD TAKE CARE STEEL CHANNELS, ANGLES, PLATES AND BARS: ..ASTM A36 2. IF CONSTRUCTED PER THESE DRAWINGS, FACILITY MEETS BLAST-RESISTANT DESIGN NOT TO OVERCOMPACT THE BACKFILL DIRECTLY BEHIND THESE RETAINING WALLS. RECTANGULAR, SQUARE, AND ROUND HSS.. ..ASTM A500. GRADE B CRITERIA FOR A 7-BAR STRUCTURAL DESIGNATION PER DOD 6055.09-M. THIS DESIGNATION STEEL PIPE (HSS). ..ASTM A53, GRADE B IN NO WAY IMPLIES VALIDATION OF THE DESIGN AGAINST OTHER LOAD CASES. Sheet reference number: 5.3 STRUCTURAL FASTENERS SHALL CONFORM TO THE FOLLOWING ASTM DESIGNATIONS: THREADED RODS. .ASTM A36 . THIS STANDARD DESIGN DRAWING DATED SEPTEMBER 2013, STD 421-80-09 SHEETS 1-23, **HEADED STUDS..** ..ASTM A108, GRADES 1015 TO 1020 (60 KSI TENSILE STRENGTH) UPDATE AND SUPERSEDE THE STANDARD DESIGN OF MAGAZINE, CONCRETE OVAL-ARCH Sheet <u>3</u> of <u>23</u> EARTH-COVERED, STD 33-15-74.

FINAL

RAWING

SIGN

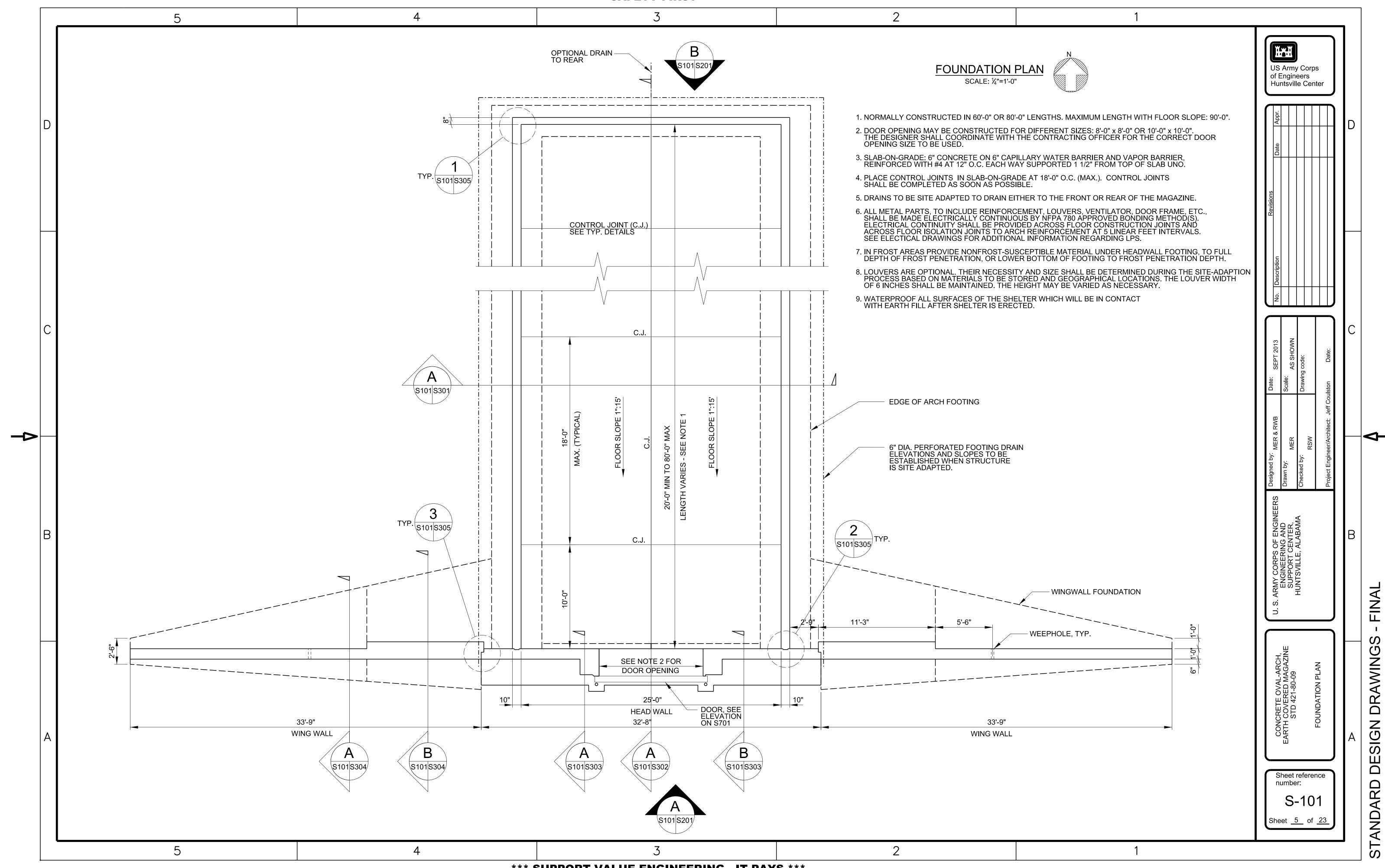
<

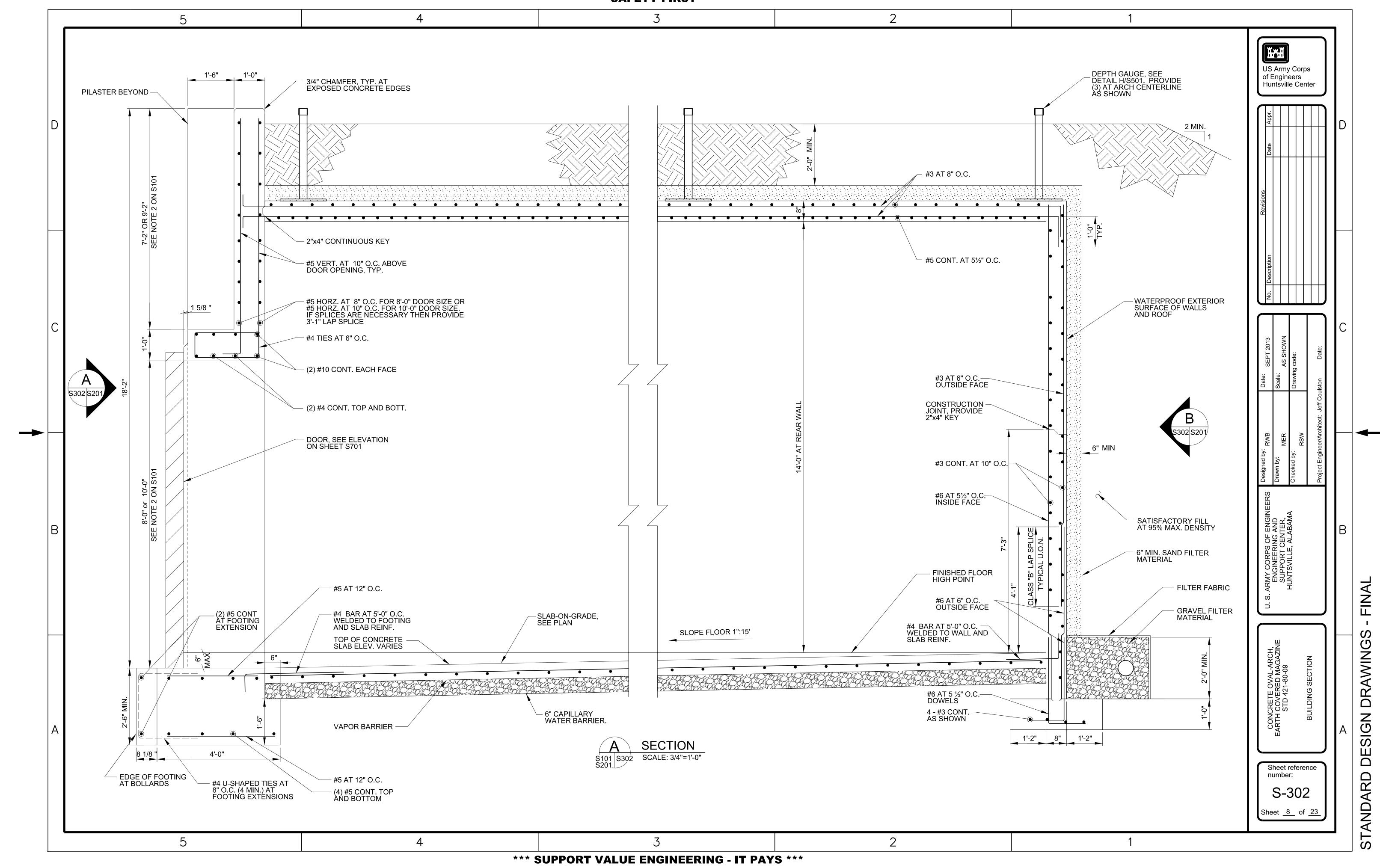
TAND,

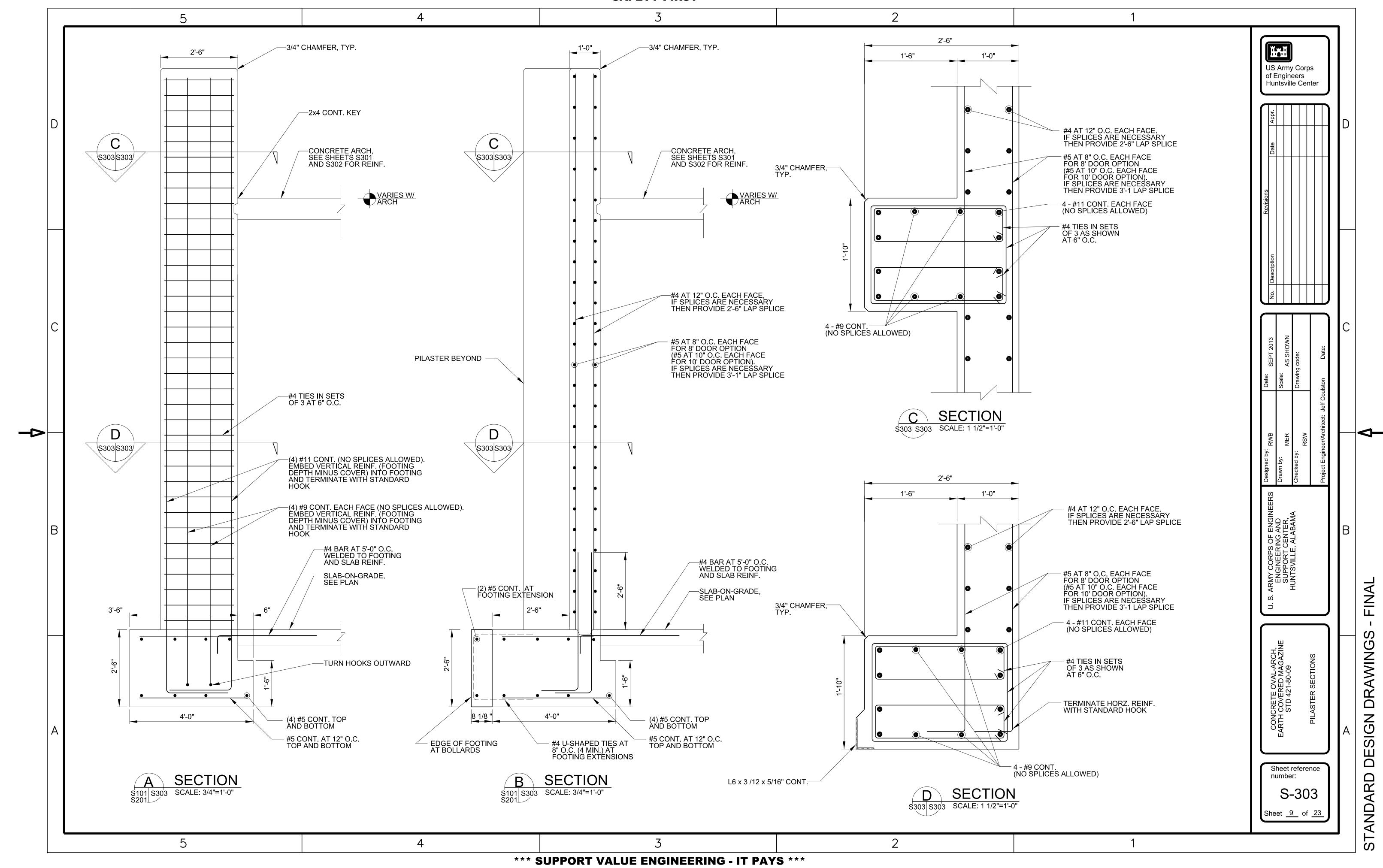
HXII US Army Corps of Engineers Huntsville Center SPECIAL INSPECTION SCHEDULE/VERIFICATION DESIGNER NOTES: TO BE REMOVED WHEN PREPARING CONSTRUCTION DRAWINGS FOR SITE ADAPTION OF THIS DESIGN. **EXTENT OF** REFERENCE COMMENTS/SCOPE INSPECTION 1 . SPECIAL INSPECTION SCHEDULE SHALL BE REVISED TO REFLECT SPECIFIC PROJECT REQUIREMENTS IN ACCORDANCE WITH CHAPTER 17 OF THE INTERNATIONAL BUILDING CODE; CONCRETE CONSTRUCTION HOWEVER, AT A MINIMUM THE SPECIAL ITEMS RELATED TO THE 'OTHER EXPLOSIVES SAFETY INSPECT SIZE, SPACING, COVER, POSITIONING AND GRADE OF REINFORCING STEEL. VERIFY THAT REINFORCING RELATED ITEMS' SHALL BE INSPECTED AS SHOWN ON THIS SCHEDULE. ACI 318: 3.5, 7.1-7.7 BARS ARE FREE OF FORM OIL OR OTHER DELETERIOUS MATERIALS. INSPECT BAR LAPS AND MECHANICAL REINFORCING STEEL PLACEMENT SPLICES. VERIFY THAT BARS ARE ADEQUATELLY TIED AND SUPPORTED ON CHAIRS OR BOLSTERS VISUALLY INSPECT ALL REINFORCING STEEL WELDS. VERIFY WELDABILITY OF REINFORCING STEEL. INSPECT AWS D1.4, C, P WELDING OF RIENFORCEMENT ACI 318:3.5.2 PREHEATING OF STEEL WHEN REQUIRED. INSPECT PLACEMENT OF CONCRETE. VERIFY THAT CONCRETE CONVEYANCE AND DEPOSITING AVOIDS С CONCRETE PLACEMENT ACI 318: 5.9, 5.10 SEGREGATION OR CONTAMINATION. VERIFY THAT CONCRETE IS PROPERLY CONSOLIDATED ASTM C 172 С SAMPLING AND TESTING OF CONCRETE ASTM C 31 TEST CONCRETE COMPRESSIVE STRENGTH, SLUMP, AIR-CONTENT AND TEMPERATURE ACI 318: 5.6, 5.8 ACI 318: 5.11-5.13 INSPECT CURING, COLD WEATHER PROTECTION AND HOT WEATHER PROTECTION PROCEDURES CURING AND PROTECTION Р FORMWORK ACI 318: 6.1.1 INSPECT FORWORK FOR SHAPE, LOCATION, AND DIMENSIONS OF THE CONCRETE MEMBER BEING FORMED DOOR CONSTRUCTION FABRICATOR CERTIFICATION/QUALITY CONTROL REVIEW OF FABRICATOR'S QUALITY CONTROL PROCEDURES OR AISC CERTIFICATION **PROCEDURES** INSPECT IN-PLANT FABRICATION, OR REVIEW FABRICATOR'S APPROVED INDEPENDENT INSPECTION AGENCY'S FABRICATOR INSPECTION REPORTS SPECIAL ITEMS RELATED TO THE BLAST STRUCTURAL STRENGTH DESIGNATION INSPECT REINFORCING STEEL TO ENSURE ELECTRICAL CONTINUITY BETWEEN THE ARCHED ROOF, WALLS, SLAB DWGS E-101; E-102 REBAR FARADAY-SHIELD AND FOUNDATION THROUGH BONDING WELDS. DOCUMENT BONDS WITH PHOTOS AND CONTINUITY TEST. VISUALLY INSPECT TO ENSURE ECM DOOR AND FOUNDATION IS BONDED TO THE GROUNDING SYSTEM. DOCUMENT Ρ DWGS E-101; E-102 ECM GROUNDNG WITH PHOTOS. DWGS E-101; E-102, VISUALLY INSPECT GROUNDING SYSEM CONDUCTORS TO ENSURE NO DAMAGE, BREAKAGE, OR CORROSION HAS GROUNDING SYSTEM NFPA 780, OCCURRED TO THE CONDUCTORS DURING INSTALL AND BEFORE EARTH BURIAL. DA PAM 385-64, 17-27. DWGS E-101, E-102, INDIVIDUAL BONDS NFPA 780, 8.9 INSPECT ALL BONDS FOR LOOSE CONNECTIONS THAT MIGHT RESULT IN HIGH-RESISTANCE CONNECTIONS. DA PAM 385-64, 17.27. NFPA 780, 8.9 INSPECT LPS COMPONENTS FOR SECURE MOUNTING AND PROTECTION AGAINST ACCIDENTAL MECHANICAL LPS COMPONENTS DA PAM 385-64, 17-27. DISPLACEMENT. NFPA 780, 8.9 PERFORM BONDING TEST ACROSS EACH BOND, AND AN EARTH ELECTRODE TEST OF THE LPS. LPS TESTING DA PAM 385-64, 17-28. INSPECT DEPTH GAUGES ON ROOF PRIOR TO EARTH COVER PLACEMENT FOR SIZE AND STABILITY. INSPECT EARTH EARTH COVER DWGS S-301-302 COVER DEPTH AND SLOPE TO ENSURE A 2' MIN. IS PROVIDED ABOVE STRUCTURE **SPECIAL INSPECTION NOTES:** INSPECTION INTERVALS ARE AS FOLLOWS: C - Continuous: The full-time observation of work requiring special inspection by an approved special inspector who is present in the area where the work is being performed P - Periodic: The part-time or intermittent observation of work requiring special inspection by an approved special inspector who is present in the area where the work has been or is being performed and at the completion of the **S** - Submittal 2 STRUCTURAL TEST AND SPECIAL INSPECTIONS ARE BASED ON CHAPTER 17 OF THE IBC 2009 EDITION CONTRACTOR SHALL HIRE A QUALIFIED INSPECTION AND TESTING AGENCY TO PERFORM SPECIAL INSPECTIONS AND TESTING IN ACCORDANCE WITH THE IBC. SUBMIT INSPECTION REPORTS TO THE CONTRACTING OFFICER FOR EACH DAY SPECIAL INSPECTIONS AND TESTING IS PERFORMED. number: Sheet <u>4</u> of <u>23</u>

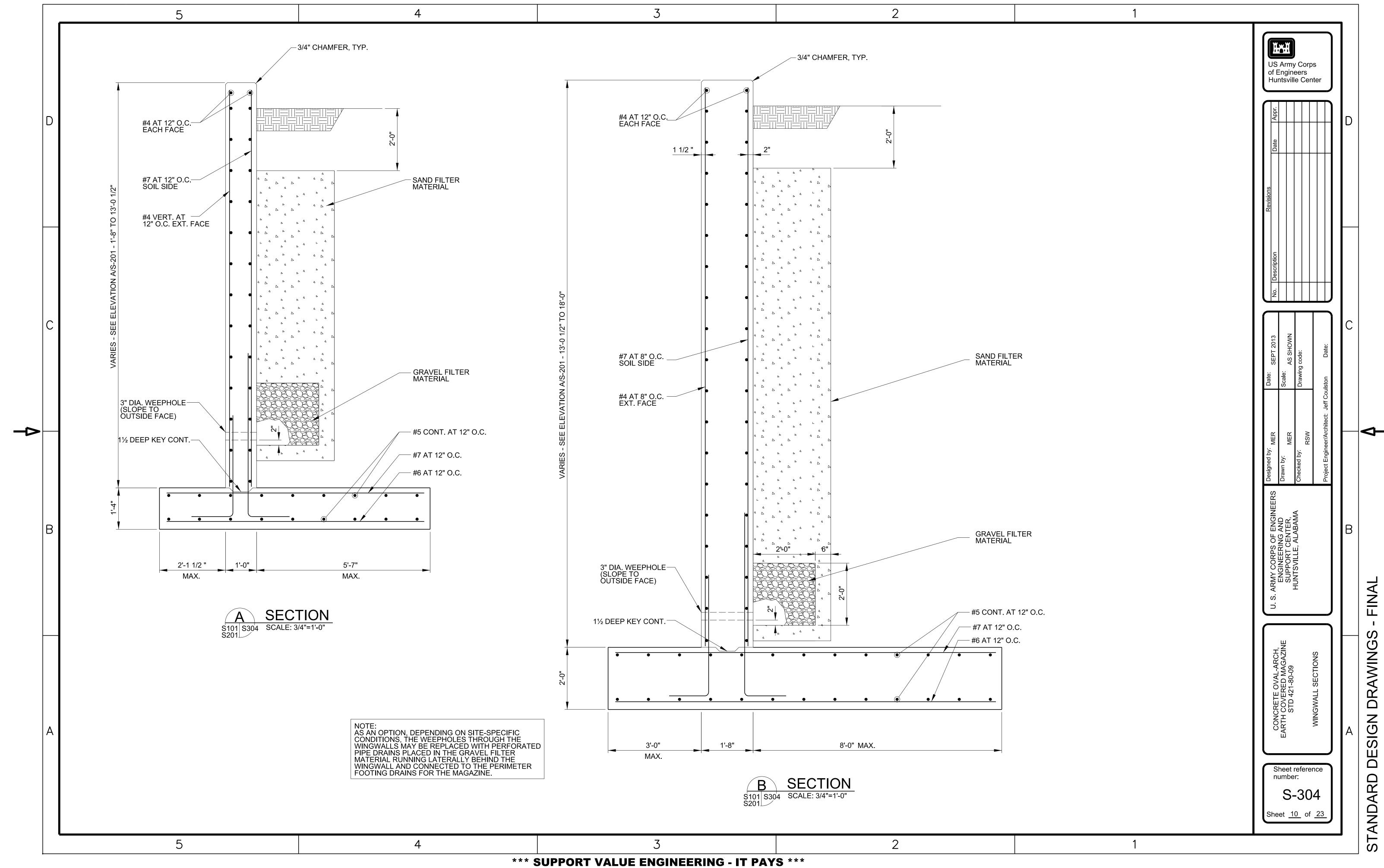
DESIGN

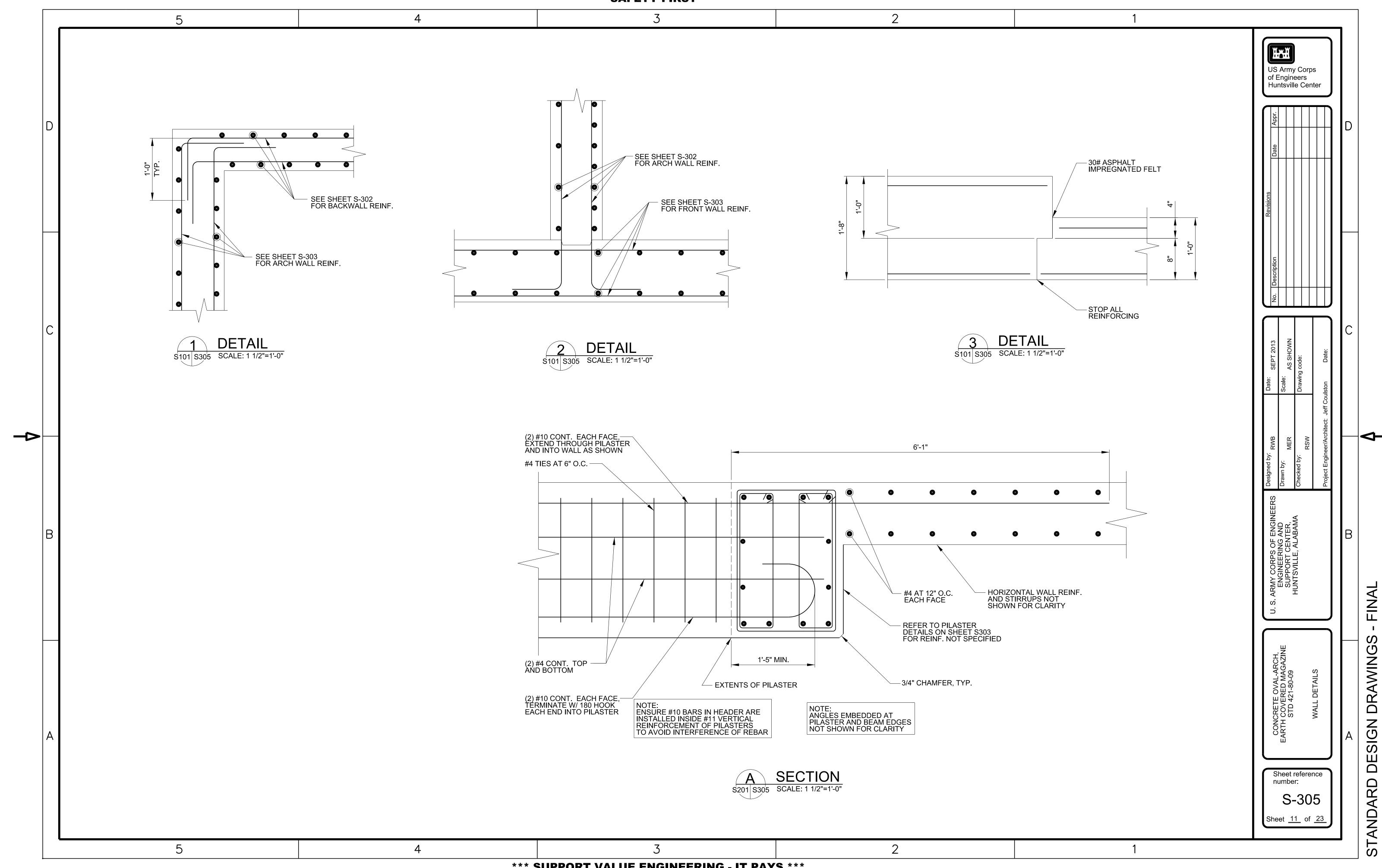
STANDARD

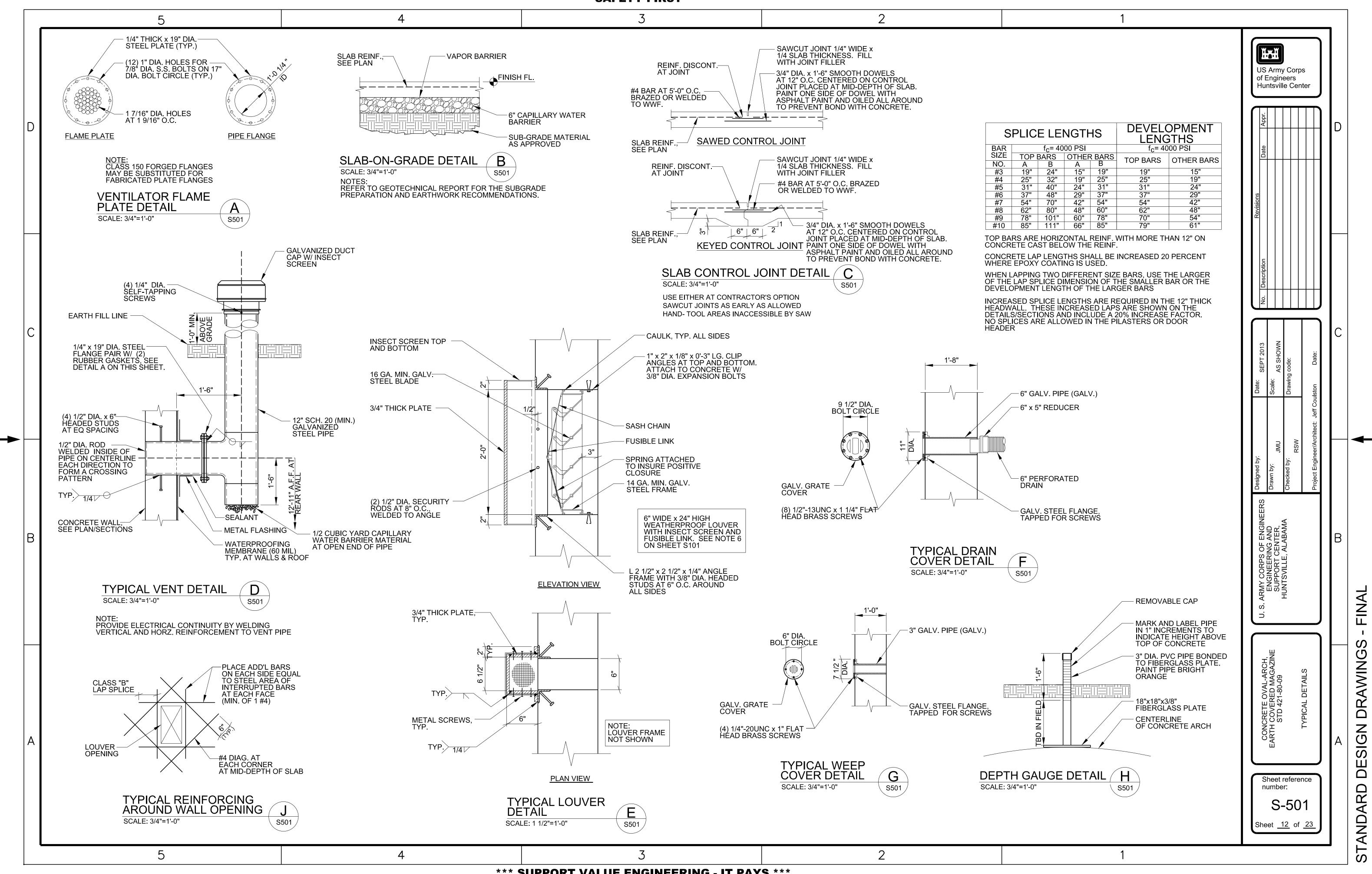






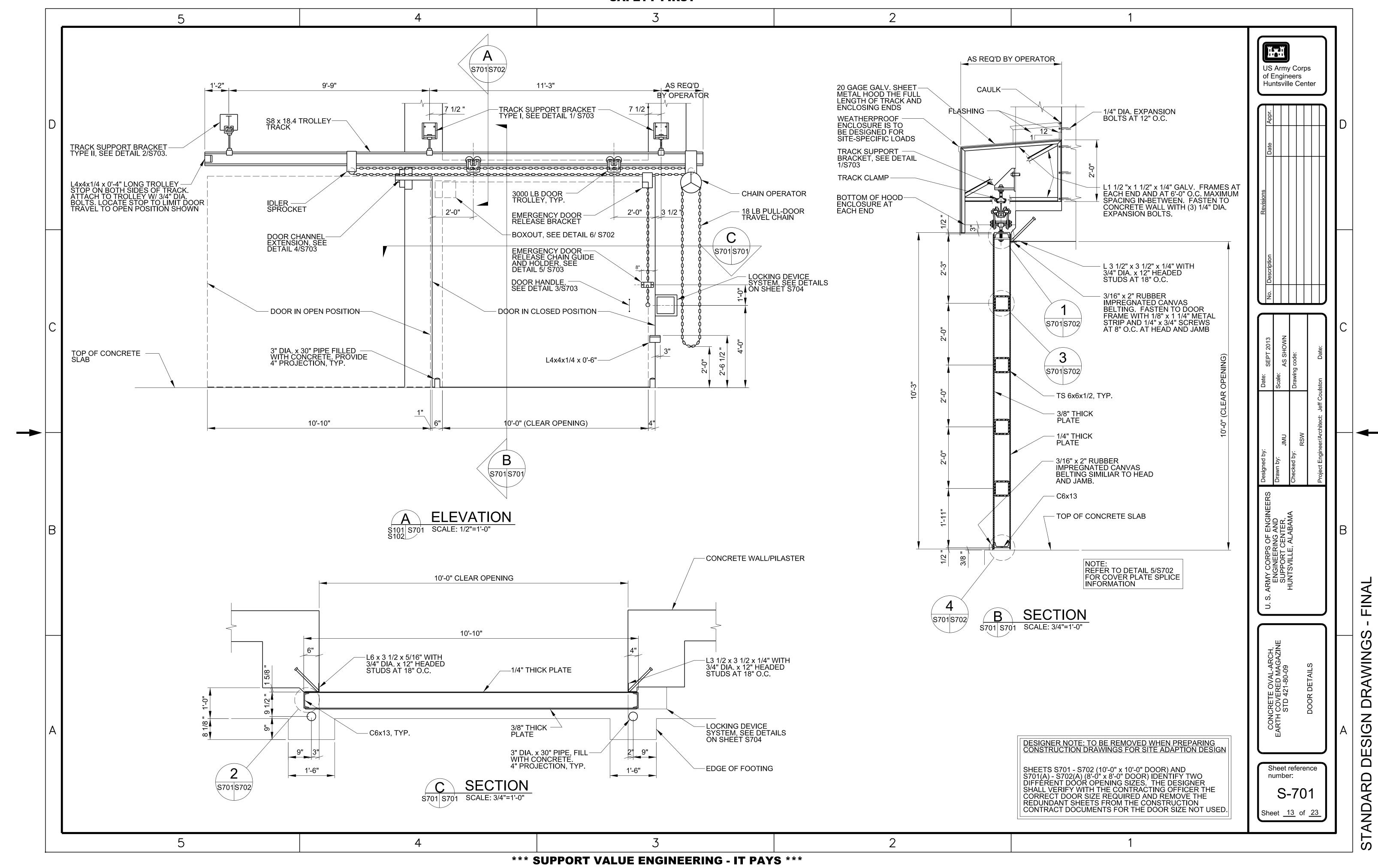


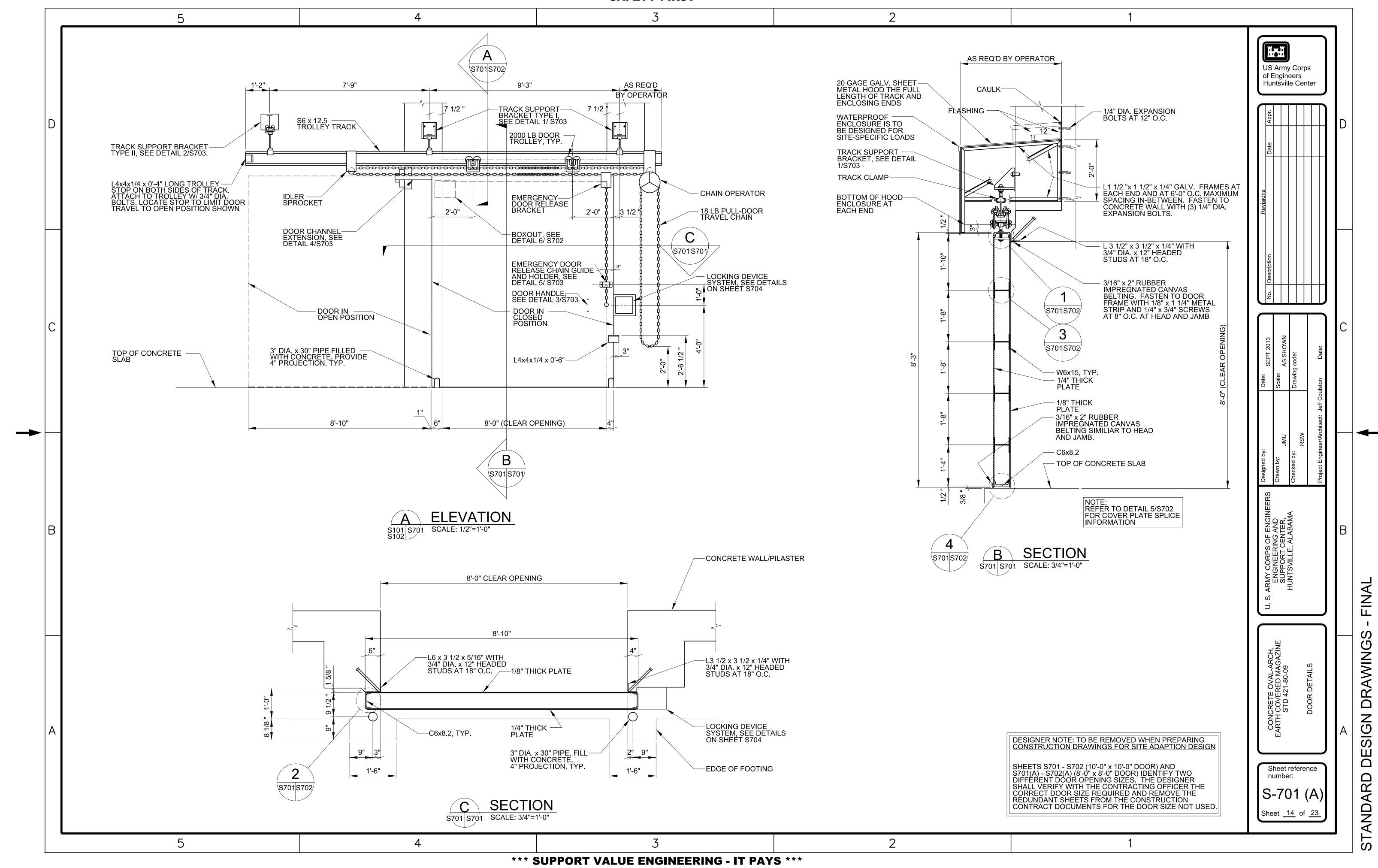


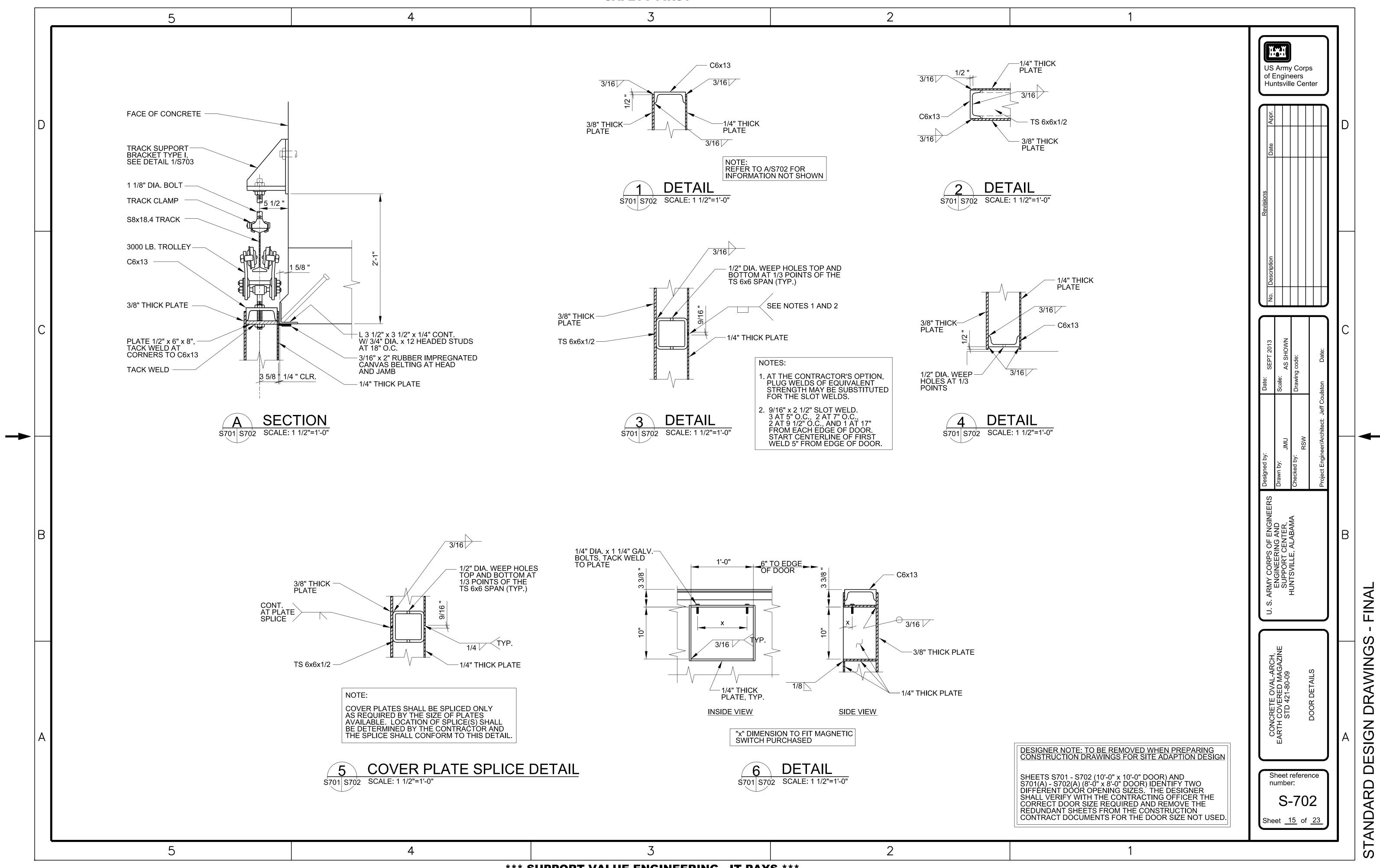


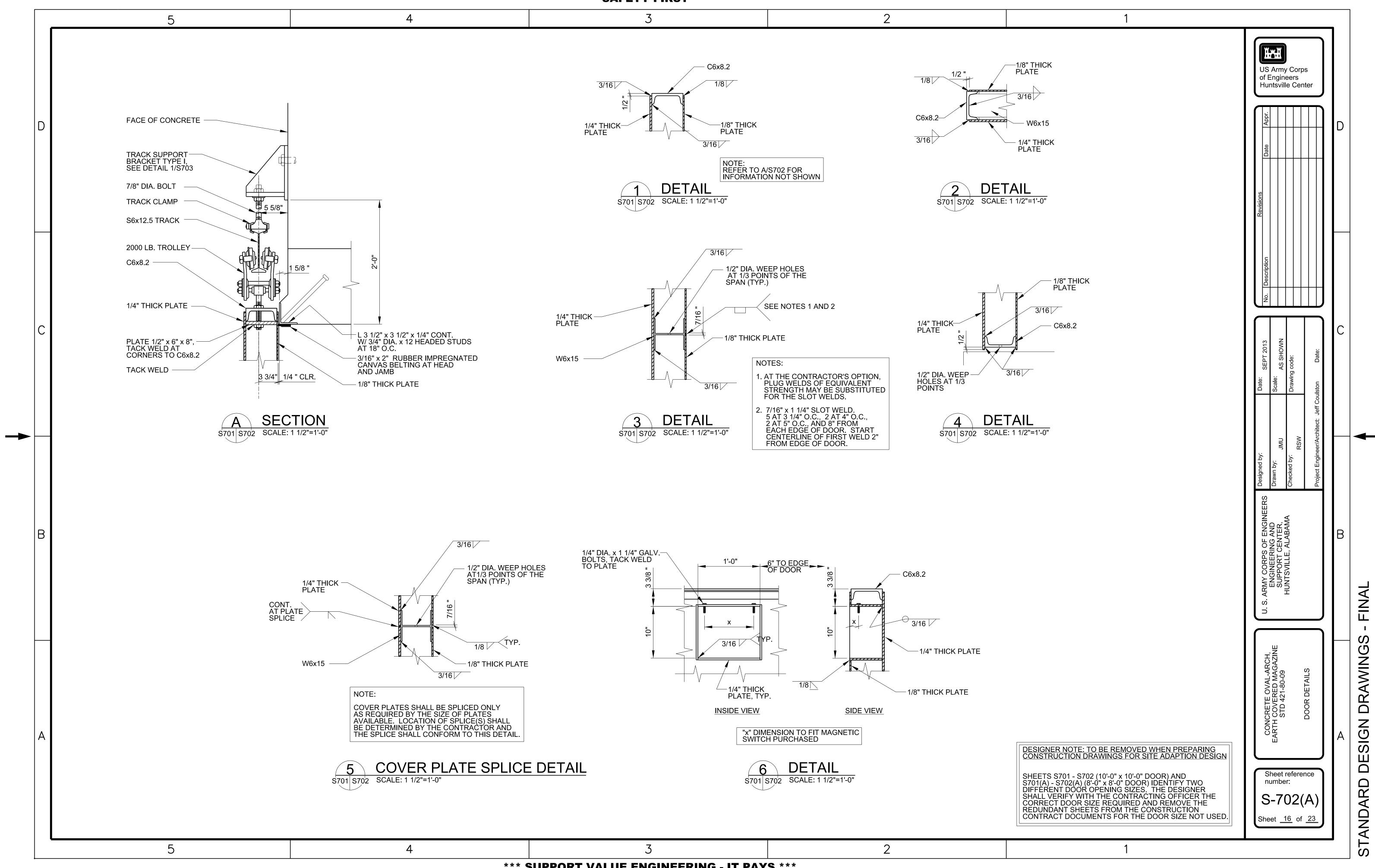
DESIGN

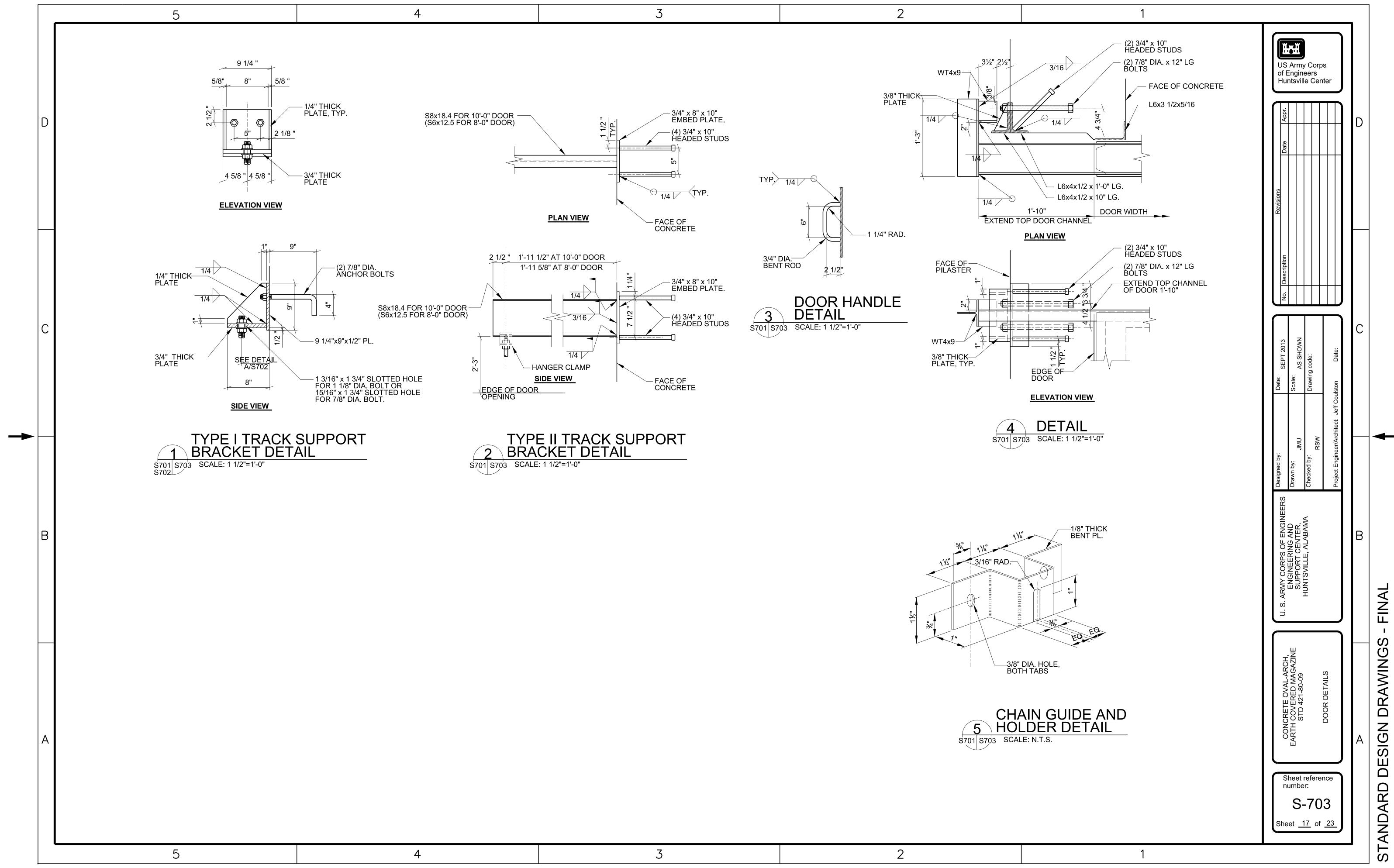
STANDARD

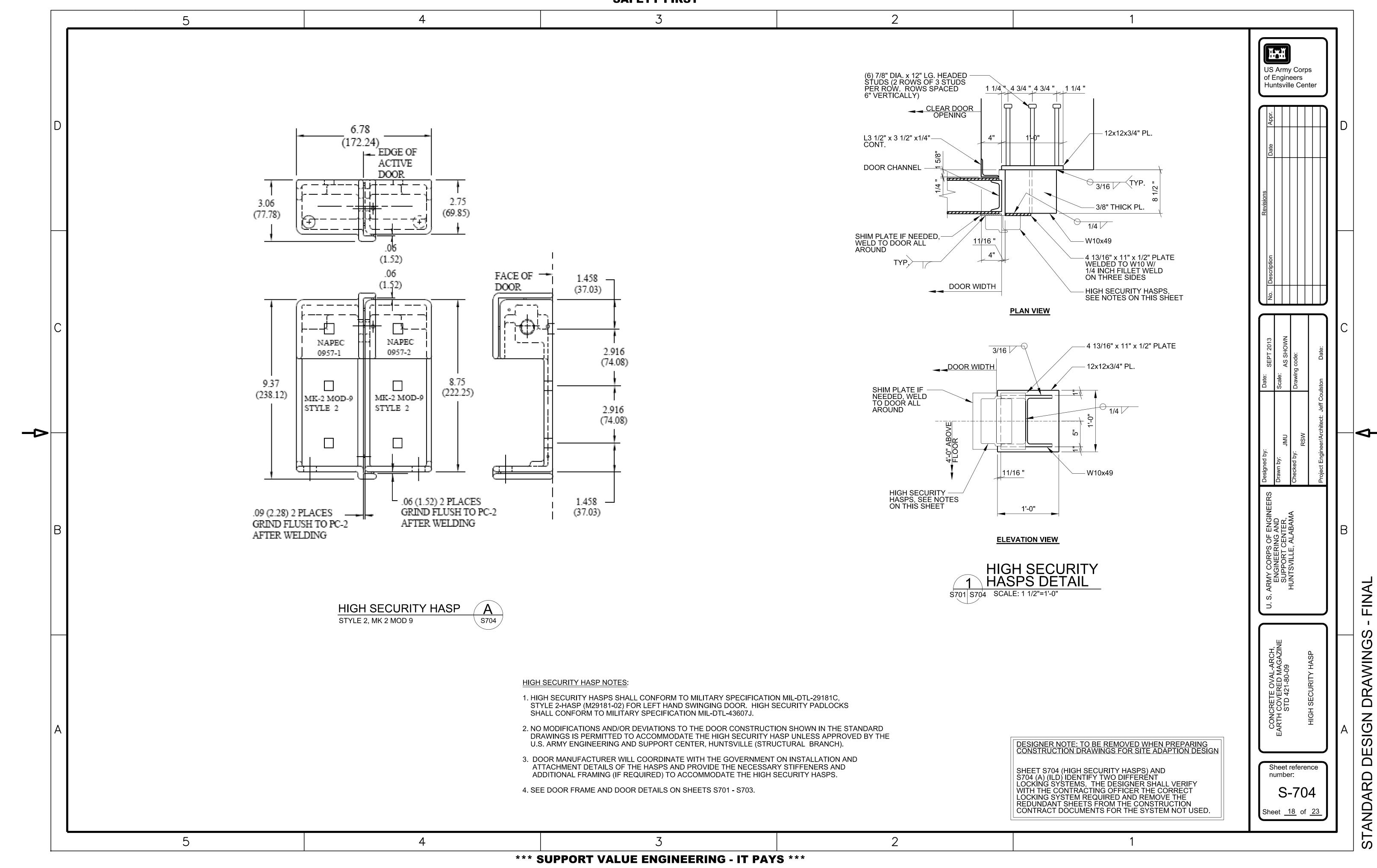


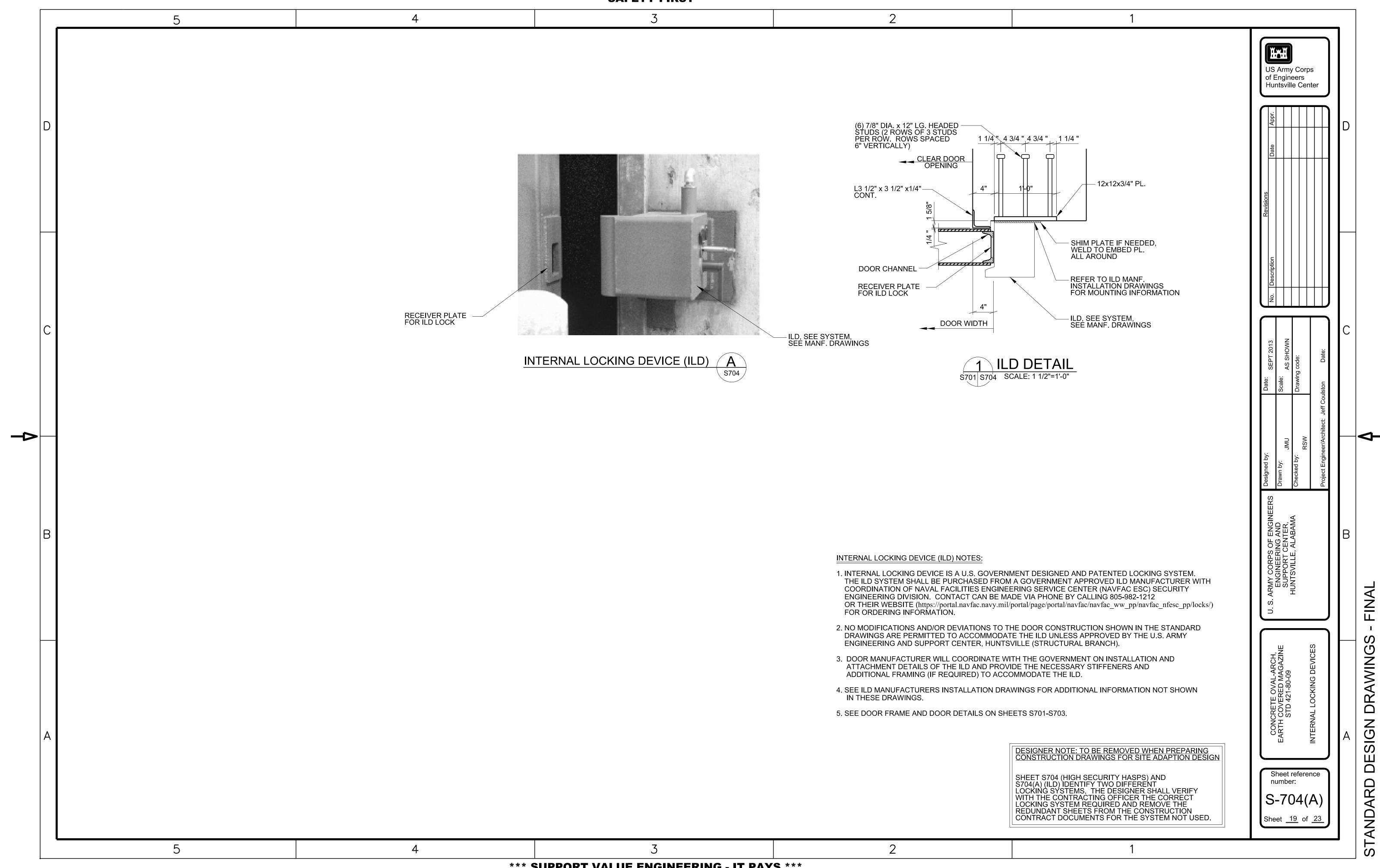


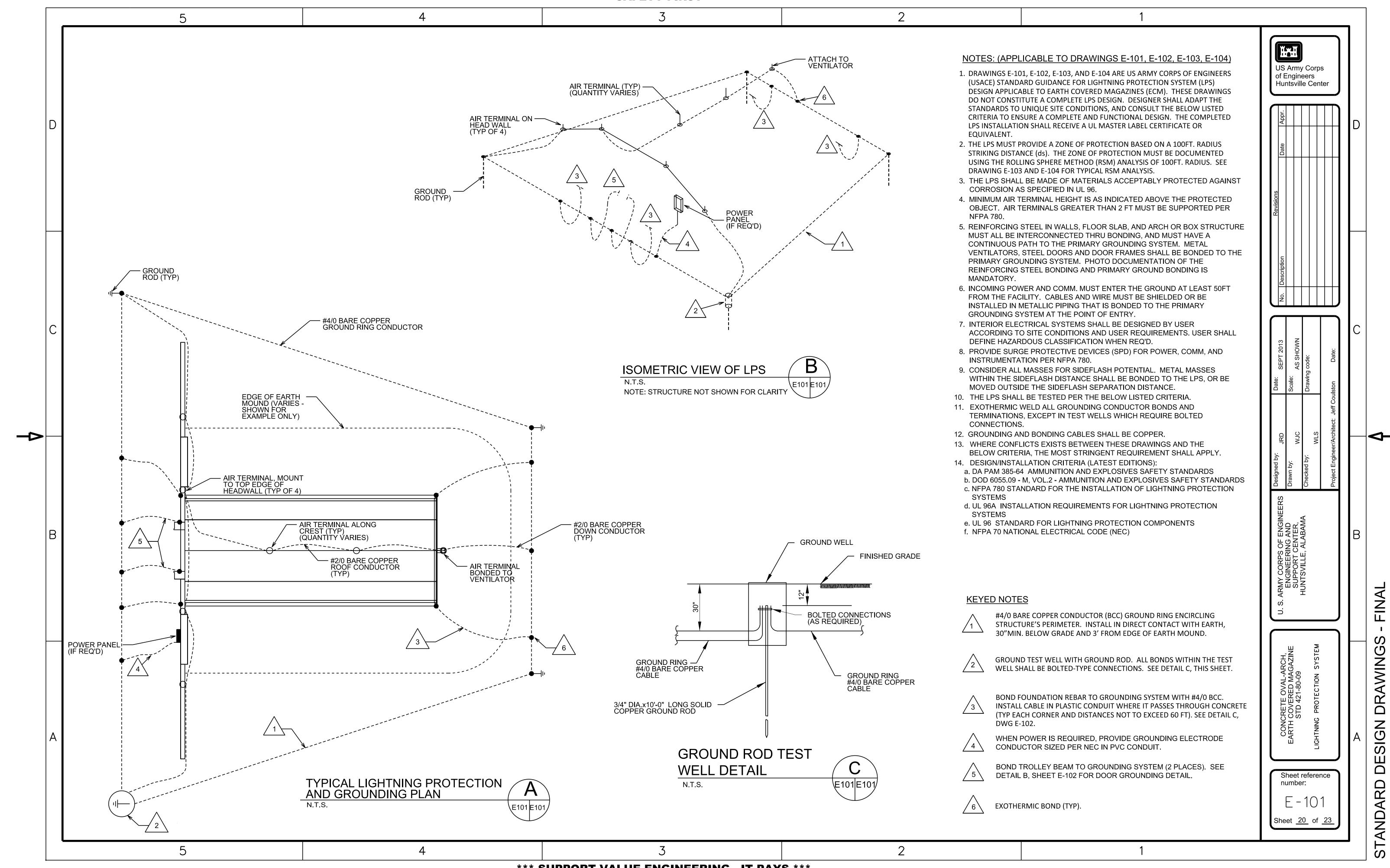












SIGN

TANDARD

